

Comparison of the Design of Water Treatment Plant by Manual and by Software Method

S. P. Sharma, S. D. Kolte, N. S. Marape, S. D. Darshanwad, S. K. Dongare, H. S. Kumawat

Abstract— The primary objective of any water supply scheme is to provide safe and adequate water supply to the area for which it is designed. Water treatment plant is the key component of such a water supply scheme, which transforms the raw water into potable water by using the appropriate treatment processes. The selection of treatment processes depends upon the raw water quality and the finished water quality objectives. The design of components of water treatment plant, construction together with good operation is very essential for water treatment plant.

In this project an attempt is made to design the conventional Water Treatment Plant of capacity is 100 MLD by manually and also by using the software. All the components of water treatment plant are included in this design. The results of design obtained by using manual method are compared with results of software method. The comparative study shows that which method is very accurate, easy and useful for the design of water treatment plant.

Index Terms— Aerator, Chlorination, Clariflocculator, Water treatment plant, WTPSOFT02

I. INTRODUCTION

Water treatment is the process of removing contaminants from raw water. It includes physical, chemical, and biological processes to remove physical, chemical and biological contaminants.

A. Classification of Most Common Water Treatment Plants

Water treatment plants can be classified as:

1. Simple disinfection

It is a direct pumping and chlorine injection, used to treat high quality water.

2. Traditional filtration plants (surface water)

Removes color, turbidity, taste, odor, and bacteria (sedimentation+ filtration plant)

3. Direct filtration plants (Surface water)

If the source water has better quality with lower solids, flocculation and sedimentation can be omitted; this modification is called direct filtration plant.

4. Softening plants (ground water) [1].

B. Objectives of Project

Its objective is to provide potable water that is chemically and biologically safe for human consumption. It should also be free from unpleasant tastes and odors and to produce both "Potable" and "Palatable" water.

To design the conventional water treatment plant with more accuracy, without mistake and in less time. To compare the design of conventional water treatment plant by manual and by software method.

C. Overview of the Water Treatment Plant, Process and Design

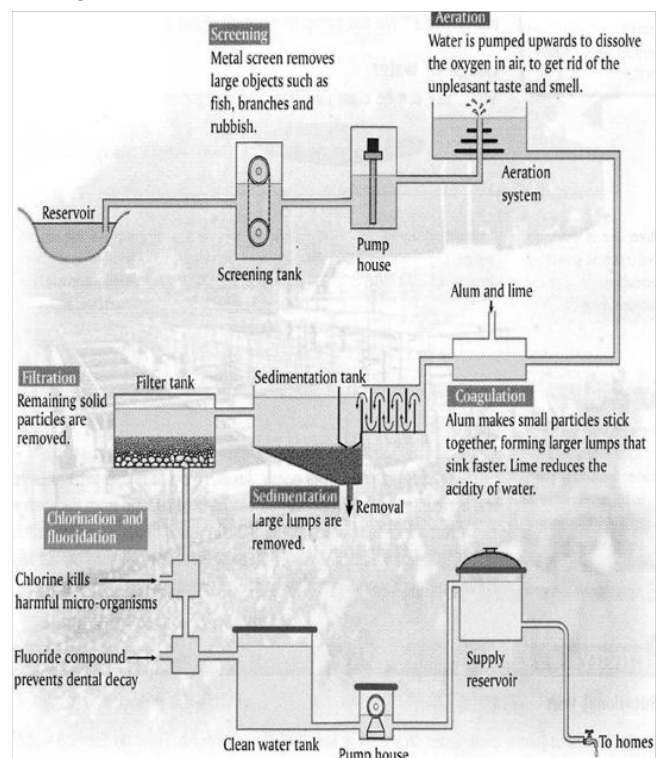


Fig.1 Water Treatment Process

The physical and chemical Quality of drinking water should not exceed the limits as per Drinking Water Quality standards IS-10500 (1991) [1, 2]. Water treatment plant is of 100 MLD capacity.

D. WTPSOFT 02 Software

Wtpsoft02 software is developed for the design of conventional water treatment plant. The design limit of software is 1MLD to 100

MLD.

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The WTP02 Design Software can be used as a tool in the hands of Design Engineer. It gives the dimensions of various units of the treatment plant and their power requirement. Engineers can use this software to increase the productivity and profitability. Academic institutions can also use this software for teaching the students the approach for the design of WTP and also for their consultancy cell [3].

II. SYSTEM DEVELOPMENT

Manual design of following components and its comparison with WTPSOFT02 Software

1. Cascade Aerator
2. Flash Mixer
3. Flocculator
4. Clarifier
5. Rapid Sand Filter
6. Parshall Flume
7. Chemical Storage Requirement
8. Chlorine and Underground Reservoir Requirement

The design of water treatment plant is done by conventional method of water treatment plant design by assuming some constant values these values are shown in result table. All the data are then compared with WTPSOFT02 Software, the input is putted into the software and its output and input is compared with conventional design.

III. RESULT AND DISCUSSION

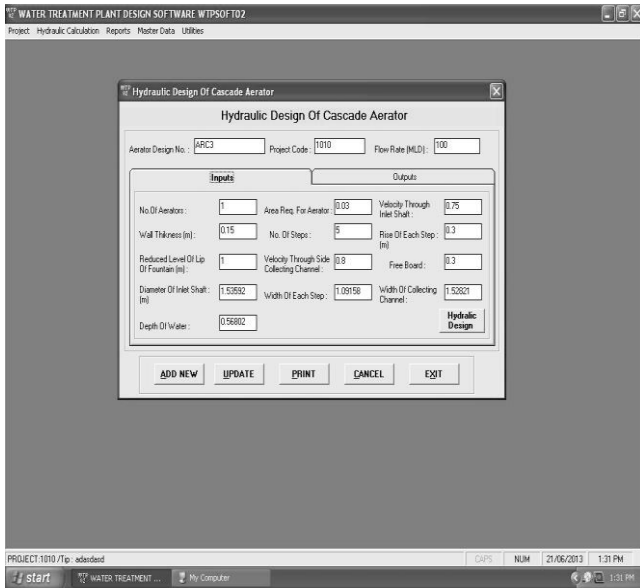


Fig. 2 hydraulic design of Cascade Aerator-input

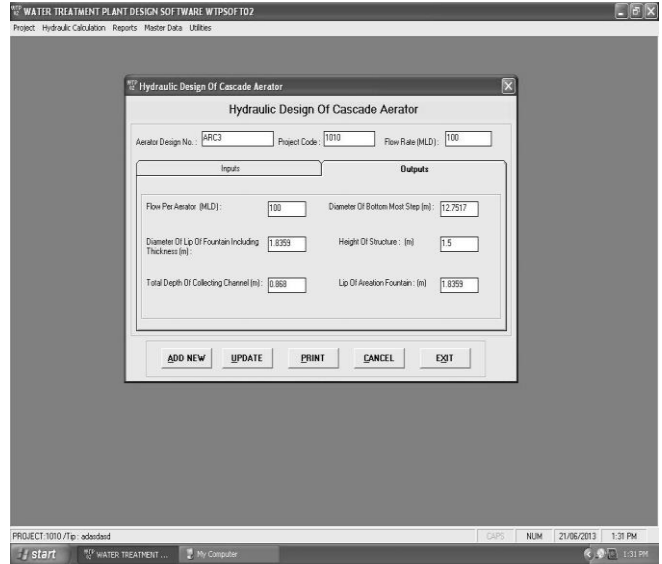


Fig. 3 hydraulic design of Cascade Aerator-output

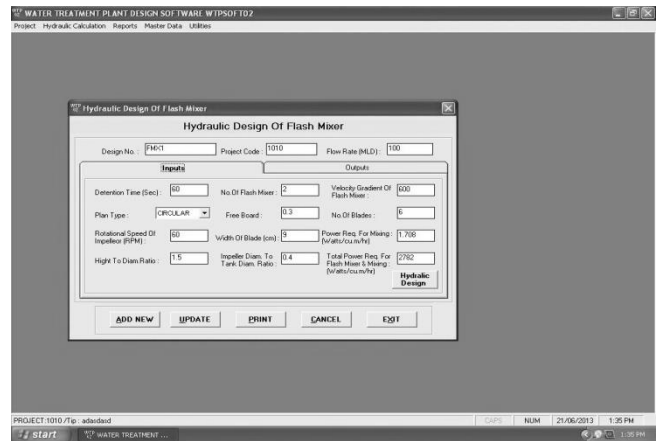


Fig. 4 Hydraulic Design of Flash Mixer-input

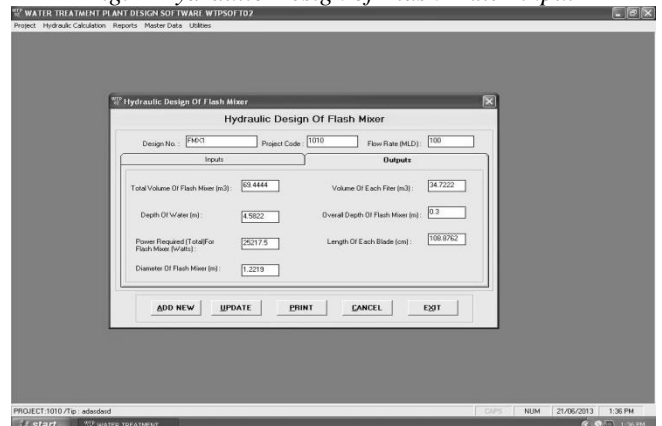


Fig. 5 Hydraulic Design of Flash Mixer-output

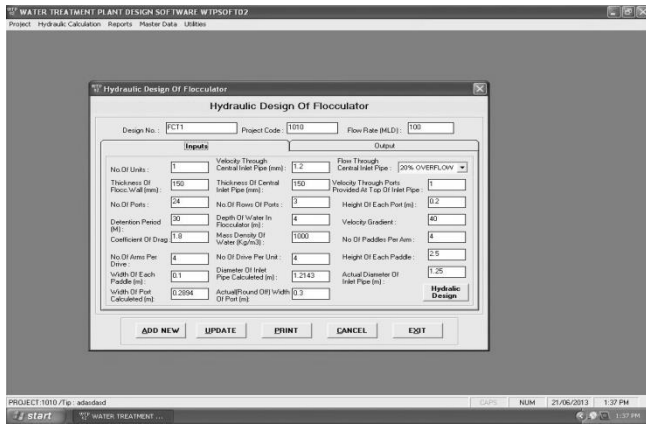


Fig. 6 Hydraulic Design of Flocculator-input

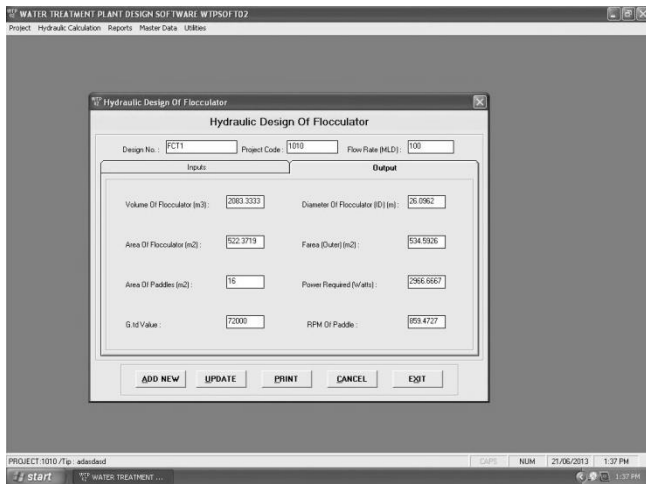


Fig. 7 Hydraulic Design of Flocculator-output

Fig. 2, 3 shows the input which is required for design of aerator are given to the software, and their respective output is shown. Likewise the different units/components are shown with their inputs and outputs in fig. 4, 5, 6 and 7.

Table -I Design summary

| Sr. No | Unit | By Manual Design | By Software Design |
|--------|---|------------------|--------------------|
| 1 | Cascade Aerator | | |
| | Inputs | | |
| | No. of Aerator | 1 | 1 |
| | Area (m ²) | 0.03 | 0.03 |
| | Velocity (m/s) | 0.75 | 0.8 |
| | No. of steps | 5 | 5 |
| | Rise of each step (m) | 0.3 | 0.3 |
| | Reduced Level of lip of fountain (m) | 1 | 1 |
| | Diameter of inlet shaft (m) | 1.6 | 1.5359 |
| | Width of each step (m) | 1.1 | 1.0916 |
| | Width of connecting channel (m) | 1.5 | 1.5282 |
| | Depth of water (m) | 0.6 | 0.5680 |
| | Outputs | | |
| | Flow per Aerator (m ³ /sec) | 1.39 | 1 |
| | diameter of bottom most steps (m) | 12.74 | 12.7517 |
| | Diameter of lip of fountain including thickness (m) | 1.8 | 1.8359 |
| | Height of structure (m) | 1.5 | 1.5 |
| | Total depth of collecting channel (m) | 0.9 | 0.868 |
| | Lip of aeration Fountain (m) | 1.8 | 1.8359 |
| 2 | Connecting Channel | | |
| | Inputs | | |
| | Channel No. | 1 | 1 |
| | Flow Rate (MLD) | 100 | 100 |
| | Length of connecting channel (m) | 5 | 5 |
| | Velocity (m/s) | 0.8 | 0.8 |

| | | | |
|---|--|---------|----------|
| | Width (m) | 0.83 | 0.8 |
| | Computed Depth of water (m) | 1.5 | 1.8085 |
| | Actual Depth of water (m) | 1.8 | 1.8085 |
| | Free Board (m) | 0.3 | 0.3 |
| | Outputs | | |
| | Total Depth (m) | 2.29 | 2.1084 |
| | Hydraulic flow in connecting channel | 1.8 | 1.811 |
| | Slope (m) | 0.0005 | 0.0005 |
| | Reduced level of Connecting channel | | -0.0025 |
| 3 | Flash Mixer | | |
| | Inputs | | |
| | Flow Rate (MLD) | 100 | 100 |
| | Detention time (sec) | 60 | 60 |
| | No of Flash Mixer | 2 | 2 |
| | Velocity (m/s) | 0.6 | 0.6 |
| | Free Board (m) | 0.3 | 0.3 |
| | No of Blades | 6 | 6 |
| | Rotational Speed of impeller (rpm) | 60 | 60 |
| | Width (cm) | | 9 |
| | Power req. For mixing (Watts/cu.m/hr) | 1.7 | 1.708 |
| | Height to Diameter ratio | 1.5 | 1.5 |
| | Impeller dia. to tank dia. ratio | 0.4 | 0.4 |
| | Total power Req. for Flash Mixer & Mixing (kW) | 3 | 2.782 |
| | Outputs | | |
| | Volume (m ³) | 69.46 | 69.4444 |
| | Volume of each filter (m ³) | 34.73 | 34.7222 |
| | Depth of water (m) | 4.65 | 4.5822 |
| | Overall depth of Flash Mixer (m) | 0.3 | 0.3 |
| | Power Required(kW) | 2.5 | 2.5217 |
| | Length of each Blade (m) | 100 | 108.8762 |
| | Diameter (m) | 1.55 | 1.2219 |
| 4 | Flocculator | | |
| | Inputs | | |
| | No. of units | 1 | 1 |
| | Velocity through central inlet pipe (m/s) | 1.2 | 1.2 |
| | Flow through central inlet pipe (overflow) | 20% | 20% |
| | No. of ports | 24 | 24 |
| | No. of rows of ports | 3 | 3 |
| | Height of each ports (m) | 0.2 | 0.2 |
| | Detention period (min.) | 30 | 30 |
| | Depth of water in Flocculator (m) | 4 | 4 |
| | Velocity gradient (m/s) | 40 | 40 |
| | No. of paddles per arm | 4 | 4 |
| | No. of arm per drive | 4 | 4 |
| | No. of drive per unit | 4 | 4 |
| | Height of each Paddle (m) | 2.5 | 2.5 |
| | Actual diameter of inlet pipe (m) | 1.25 | 1.25 |
| | Width of port (m) | 0.3 | 0.3 |
| | Actual round of width of port (m) | 0.3 | 0.3 |
| | Outputs | | |
| | Volume of flocculator (m ³) | 2084.44 | 2083.333 |
| | Diameter (m) | 18.25 | 26.0962 |
| | Area (m ²) | 521.1 | 522.3719 |
| | Area of paddles (m ²) | 16 | 16 |
| | Power reqd.(kW) | 2.968 | 2.9667 |
| 5 | Clarifier | | |
| | Inputs | | |
| | No. of unit | 2 | 2 |
| | Detention period (hr) | 2.5 | 2.5 |
| | Depth (m) | 3 | 3 |
| | Max. overflow rate (m ³ /m ² .d) | 36 | 35 |
| | Dia. of Flocculator (m) | 18.25 | 18.25 |
| | Velocity of launder (m/s) | 0.6 | 0.6 |
| | Width of launder (m) | 0.8 | 0.8 |
| | Outputs | | |



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| | | | |
|----------|---|-------|----------|
| | Dia. of clarifier (m) | 50.6 | 50 |
| | Depth of clarifier at center (m) | 5.8 | 5.8073 |
| | Weir loading (m ³ /d/m) | 300 | 382.1656 |
| | No. of projection | 14 | 14 |
| | Launder depth (m) | 1.024 | 1.0234 |
| | No. of orifices | 530 | 666.6667 |
| | No. of V-notches | 299 | -- |
| 6 | Rapid Sand Filter | | |
| | Inputs general | | |
| | Filtration rate | 5 | 5 |
| | Length to width ratio | 1.3 | 1.3 |
| | Depth of gravel filter bed(cm) | 45 | 45 |
| | Depth of sand filter bed(cm) | 75 | 75 |
| | Depth of water over sand bed(cm) | 2 | 2 |
| | Effective size of sand (mm) | 0.5 | 0.45 |
| | Uniformity coefficient | 1.5 | 1.4 |
| | Orifice area reqd. in terms of % of filter area | 0.3 | 0.3 |
| | Under drainage system | | |
| | Rate of back washing (m/hr) | 36 | 36 |
| | Time of back washing (min) | 10 | 10 |
| | Dia. of orifice(mm) | 12 | 12 |
| | Dia. of lateral (mm) | 80 | 80 |
| | Calculation dia. of manifold | 0.7 | 0.6 |
| | No. of lateral calculated | 50 | 36 |
| | Time req. for filling wash water tank (hr) | 24 | 24 |
| | Design of wash water trough | | |
| | Max. horizontal travel of wash water | 0.9 | 0.9 |
| | Depth of water (m) | 0.4 | 0.4 |
| | % Expansion of sand bed during back washing | 50 | 50 |
| | Width of gullet (m) | 0.6 | 0.6 |
| | Free board for gullet | 0.3 | 0.3 |
| | Provide No. of through | 5 | 4 |
| | Design of wash water pump | | |
| | Wash water tank height | 8 | 8 |
| | Assume suction head | 4 | 4 |
| | Length of delivery pipe (m) | 25 | 25 |
| | Dia. of delivery pipe (mm) | 150 | 150 |
| | Efficiency of pump (%) | 70 | 70 |
| | Efficiency of motor (%) | 90 | 90 |
| | Actual provided head | 210 | 210 |
| | Actual BHP provided | 17.5 | 18.3164 |
| | Design of pipes | | |
| | No. of inlet channel to feed filter | 2 | 2 |
| | Velocity in the channel (m/s) | 1 | 1 |
| | Width of inlet channel (m) | 1 | 1 |
| | Provided dia. of inlet pipe each | 0.4 | 0.3555 |
| | Provided dia. of filter wash water | 0.4 | 0.5507 |
| | Outputs | | |
| | Depth of filter (m) | 3.7 | 3.2 |
| | Total depth of trough (m) | 0.5 | 0.55 |
| | Dist. of trough from sand bed (m) | 43 | 43.05 |
| | Interval of backwash between any two bed (hr) | 1.7 | 1.7143 |
| 7 | Flow Measurement Device | | |
| | Inputs | | |
| | Value of manning | 0.013 | 0.013 |
| | Length of channel Parshall Flume (m) | 5 | 5 |
| | Velocity through channel (m/s) | 0.8 | 0.8 |
| | Outputs | | |
| | Throat width of flume | 300 | 300 |
| | Length of conveying part (m) | 1322 | 1322 |
| | Width at diverging | 600 | 600 |
| | Width of converging | 831 | 831 |
| | Throat length of flume (m) | 600 | 600 |
| | Length of diverging part | 900 | 900 |
| | A | 1350 | 1350 |
| | K | 75 | 75 |
| | Z | 225 | 225 |
| 8 | Chemical Storage Requirement | | |
| | Inputs | | |

| | | | |
|----------|---|------|----------|
| | Average alum dose | 40 | 40 |
| | % solution strength for alum | 5 | 5 |
| | No. of tanks including 1 stand by | 3 | 3 |
| | operation time (hr) | 8 | 8 |
| | Outputs | | |
| | Qty. of alum req (kg/day) | 3111 | 4000 |
| | Volumetric capacity | | 13.33 |
| | Size of each tank | 3 | 2.108 |
| | Bag of alum req/month | 2400 | 2400 |
| | Area req. for alum storage | 1270 | 1270.588 |
| 9 | Chlorine and Underground Reservoir Requirement | | |
| | Chlorine dose | 2 | 2 |
| | Detention time | 60 | 60 |
| | Depth of reservoir (m) | 2 | 2 |
| | No. of component | 4 | 4 |
| | Length of each component (m) | 1.2 | 1.2 |
| | Width of component (m) | 1.2 | 1.2 |
| | Component area | 1.44 | 1.44 |
| | Chlorine req. (kg/d) | 200 | 200 |
| | Volume of underground clear water | 4166 | 4166.666 |
| | Area of reservoir | 2083 | 2083.333 |
| | Comp. area | 520 | 520.8334 |
| | Weight of each cylinder | 50 | 50 |

IV. CONCLUSION

The main objective of WTP is to provide potable water that is chemically and biologically safe for human consumption and it should free from unpleasant tastes and odors is fulfill by design of WTP , in which all process are included for purification of water.

The parameters or components which are needed for these processes are design by manual and software method.

By comparing the results of these two methods, we concluded that,

The results obtained by these two methods are approximately same. The manual method is lindy and tedious anther hand the software method is easy and interesting. The time consumption for design by software method is less than the manual method. The errors in manual method are more than of software method.

According to all above points, the software method is more accurate.

APPENDIX

| | |
|---------------------|------------------------|
| MLD | Million Liter per day |
| W.T.P | Water treatment plant |
| cu. m | Cubic meter |
| cm | Centimeter |
| m | Meter |
| m ² | Square Meter |
| m/s | Meter per second |
| m ³ /sec | Cubic Meter per second |
| sec | second |
| hr | hour |
| rpm | Revolution per minute |
| kW | Kilo watt |
| min. | minute |
| m/hr | Meter per hour |
| mm | Millimeter |
| kg/day | Kilogram per day |



REFERENCES

1. A. G. Bhole, "Design of Water Treatment Plants", Indian Water Works Association, Nagpur Centre, Nagpur, pp. 123-150
2. CPHEEO, *Manual of Water Supply and Treatment*, Ministry of Urban Development, New Delhi, 1999
3. Prachi services, "User guide of software", Prachi services, Jogeshwari, Mumbai.

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Prof. Hastimal S. Kumawat, Bachelor of Engineering-Civil, Master of Engineering-Civil-Water Resources Engineering, Publications- U. J. Kahalekar, H. S. Kumawat, "Evaporation Suppression from Water Surfaces using Chemical films", International conference on Sustainable Innovative Techniques in Civil and Environmental Engineering SITCEE-2013, New Delhi, India, 2013, pp.38-43., U. J. Kahalekar, H. S. Kumawat, "Evaporation Suppression from Water Surfaces using Chemical films", International Journal of Civil Engineering and Technology (IJCIET), Volume 4, Issue 3, May - June (2013), pp. 185-196, ISSN Print: 0976 – 6308, ISSN Online: 0976 – 6316. Research work- "Evaporation suppression from water surfaces using chemical films,"