

# Hydraulic Conductivity of Sandy Soil with Varying Grain Size

P. Dayakar, K. Venkat Raman, R.Venkata Krishnaiah

**Abstract -** A concept of mean particle size and size factor is presented in this paper and an attempt is made to relate the hydraulic conductivity to mean particle size, taking into account the optimum moisture content and maximum dry density. The distribution of particle size has a significant effect on hydraulic conductivity. It is possible to determine the mean particle size and size factor from the gradation curve. For the purpose of experimental analysis, three samples of soils were collected, local river sand available in market, coarse sand 1.18mm sieve and quarry dust. For determining the hydraulic conductivity a constant head permeability test is conducted. A few exact conditions to compute water powered conductivity utilizing grain estimate dispersion of unconsolidated spring materials have been assessed in this examination. Evaluating examination of soil tests extricated from the test openings during ground water examination venture was performed to decide their arrangement and molecule measure dispersion attributes, from which pressure driven conductivities were figured. In particular, every single exact equation are to be utilized carefully inside the space of appropriateness.

**Key words:** Hydraulic conductivity, Grain size analysis, Empirical formula.

## I. INTRODUCTION

Hydraulic Pressure driven conductivity is one of the most significant normal for water bearing developments. Its extent, design, changeability fundamentally impact ground-water stream design[1],[3],[5]. All in all, pressure driven conductivity speaks to capacity of a permeable medium to transmit water through its interconnected voids. Water powered conductivity of soils is a significant parameter in numerous seaward geotechnical examines, for example, wave-soil cooperation[32],[34]. Darcy's condition holds useful for the laminar progression of water through the void spaces in soil (sand and earth). In view of this examinations it has been reasoned that, for stream of water through fine and medium sand, sediment, and mud, the stream is laminar and Darcy's law is substantial (Muskat, 1937; Mitchell, 1976) [25],[27],[29].

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The index properties of the soil samples namely fine sand, coarse sand and quarry dust used in this project work is furnished in table 1.

Table 1 – Properties Of Soil Samples

Description	Fine Sand (FS)	Coarse Sand (CS)	Quarry Dust (QD)
Specific Gravity	2.67	2.60	2.76
D10	0.099	0.73	0.1
D15	0.18	0.93	0.2
D30	0.24	1.05	0.4
D60	0.36	1.1	1.8
Cc	1.6	1.767	0.88
Cu	3.6	1.833	18
$\gamma_d$ max	1.84	1.78	2.26
OMC	14.29	7.95	6.67
$e_{max}$	0.71	0.69	0.51
$e_{min}$	0.52	0.54	0.3
$e_{avg}$	0.615	0.615	0.405
Classification	SW	SP	SP

## II. GRAIN SIZE DISTRIBUTION

This arrangements with the technique for the assurance of grain estimate dispersion in soils. An examination of this caring express quantitatively the extents by mass of the different sizes of particles present in the dirt. In a dirt, the rock, sand, residue and mud divisions are perceived as containing particles of diminishing size[2],[4],[6]. The real scope of measurements of the particles are given in IS: 1498-1970. The aftereffects of grain measure examination may likewise be spoken to graphically as a grain estimate conveyance bend in which the aggregate rates better than realized proportionate grain sizes are plotted against these sizes, the last being on a logarithmic scale. The consequences of grain measure investigation are broadly utilized in soil arrangement[7],[9],[11]. The information acquired from grain measure appropriation bends is utilized in the structure of channels for earth dams to decide the reasonableness of soils for street development. In this the standard, grain sizes have been gotten line with IS: 1498-1970.

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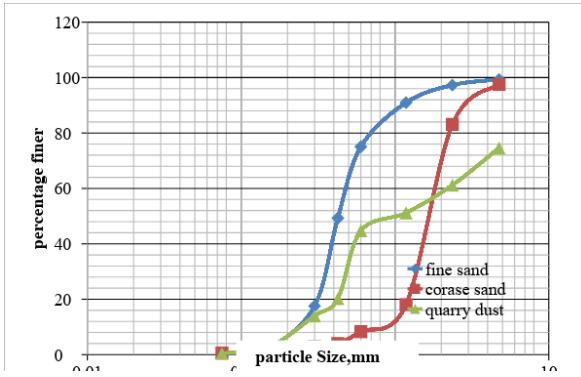


Fig. 1 Particle size distribution of soil samples

### III. METHODOLOGY

For preparing hydraulic conductivity of test setup an acrylic cylinder of Diameter 19 cm height 26 cm average flow Q 50 cm<sup>3</sup>/sec Constant Hydraulic head h, 37.5 cm.



Fig 2 Hydraulic Conductivity typical test setup

The notations followed for various soils in various states for determining the hydraulic conductivity is furnished in table 2.

Table – 2 Notation Followed

Description	Notation
	s
Fine Sand Loose State	FSLS
Fine Sand Medium Dense	FSMD
Fine Sand Dense State	FSDS
Coarse Sand Loose State	CSLS
Coarse Sand Medium Dense	CSMD
Coarse Sand Dense State	CSDS
Quarry Dust Loose State	QDLS
Quarry Dust Medium Dense	QDMD
Quarry Dust Dense State	QDDS

### IV. EXPERIMENTAL INVESTIGATION

Though the cohesionless soils have unique hydraulic conductivity, the behavior of the same is different when a combination of layers are formed[8],[10],[12]. To understand this behavior in the first case a combination of coarse sand, quarry dust and fine sand is placed one over the other and hydraulic conductivity is determined. Fig 3.15 shows the test setup of the determination of hydraulic conductivity of combination of quarry dust, c in the medium dense state. To obtain this state of soil the three samples is filled in the acrylic tube in the dense state by compacting for 10cm filled to reduce the void ratio partially[26],[28],[30]



Fig 3. Hydraulic Conductivity of layered soil coarse sand, quarry dust and fine sand and Quarry Dust, Coarse Sand , Fine Sand - Medium Dense State

A relationship between hydraulic conductivity and time taken to reach a constant value of hydraulic conductivity for the coarse sand, quarry dust and fine sand in the medium dense and state is shown in fig 4[31],[33]. In this case, approximately after 480 seconds a constant value is established, and the quarry dust, coarse sand and fine sand in the medium dense state is in this case, approximately after 1530 seconds a constant value is established[14],[16], [18].

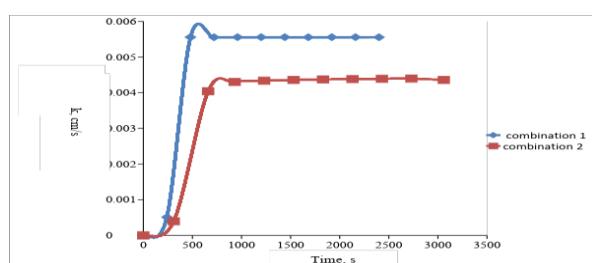


Fig 4. Relationship between Hydraulic Conductivity and Time for Coarse Sand, Quarry Dust and Fine Sand and quarry dust, coarse sand and fine sand - Medium Dense State.

## V RESULT AND DISCUSSION

From the experimental investigation of determining hydraulic conductivity for various states of different soil samples, a comparison is made with the available empirical relationship to determine the hydraulic conductivity[19],[21],[23]. Table 3 shows the comparison of hydraulic conductivity determined by empirical relationship namely Allen Hazen's equation and Sherard's equation along with the experimental values.

Table 3. shows the comparison of hydraulic conductivity of three different soil samples in three different states namely loose, medium and dense state[13], [15] ,[17]. From the table, it can be understood that for any type of soil loose state gives higher hydraulic conductivity when compare to dense state.

From the investigation, it can be seen that quarry dust in the loose state is more porous then coarse sand and fine sand. A graphical representation of the same is shown in the fig 5. to understand the behavior clearly [20],[ 22], [24]

Table – 3 Comparision of Empirical Formula and Experimental

Value

Soil	State	Experime ntal value k, cm/s	Allen Hazen's Equation k, cm/s	Sherard's Equation k, cm/s
Equatio n		$k=QL/Ah$ $t$	$k=C(D10)^2$	$k=0.35(D15)^2$
Fine Sand	Loos e	0.1858	0.0225	0.011
Fine Sand	Medi um	0.0776		
Fine Sand	Dens e	0.0173		
Coarse Sand	Loos e	0.1957	0.5329	0.5
Coarse Sand	Medi um	0.1638		
Coarse Sand	Dens e	0.1101		
Quarry Dust	Loos e	0.2755	0.01	0.014
Quarry Dust	Medi um	0.0024		
Quarry Dust	Dens e	0.0005		
C.S+Q. D+F.S	Medi um	0.0045	-	-
Q.D+C. S+F.S	Medi um	0.0035	-	-

A comparison is made for the various cases of determination of hydraulic conductivity by plotting the curve in a single graph as shown in fig 5.

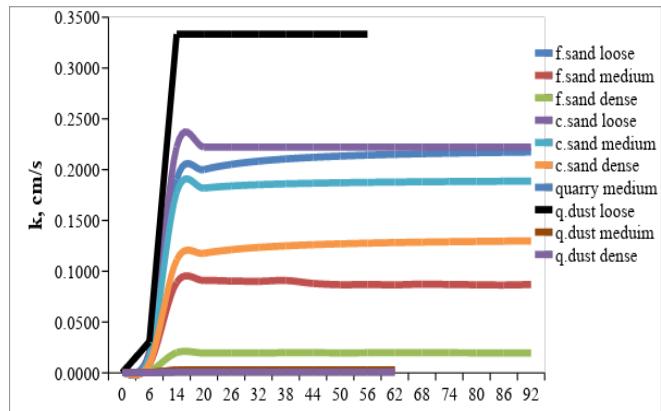


Fig 5. Relationship between HydraulicConductivity and Time for all the three soil samples

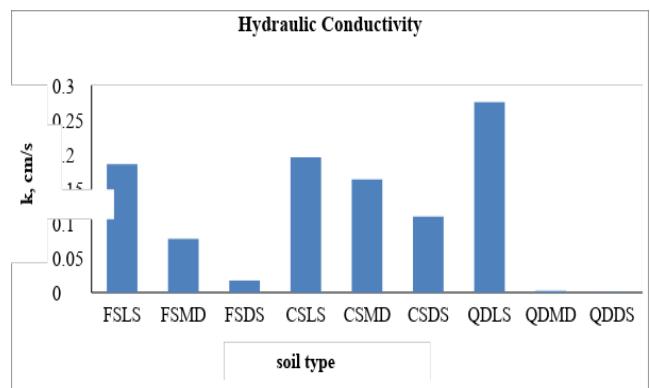


Fig 6 Graphical representation of Hydraulic Conductivity vs soil samples

## V. CONCLUSION

In case of fine sand there is a reduction of 90% and 58% hydraulic conductivity when dense state and the medium dense state is compared with loose state respectively .

In case of coarse sand there is a reduction of 43% and 16% hydraulic conductivity when dense state and the medium dense state is compared with loose state respectively .

In case of quarry dust there is a reduction of 99% and 99% hydraulic conductivity when dense state and the medium dense state is compared with loose state respectively.

From the comparison of experimental results with the empirical relationship it may be concluded that only overestimation or underestimation of hydraulic conductivity can be arrived when these empirical relationship are used.

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## REFERENCES

1. Iyappan L., Dayakar P., Identification of landslide prone zone for coonoortalukusing spatial technology, International Journal of Applied Engineering Research,V-9,I-22,PP-5724-5732,Y-2014.
2. Kumar J., Sathish Kumar K., Dayakar P.,Effect of microsilica on high strength concrete, International Journal of Applied Engineering Research,V-9,I-22,PP-5427-5432,Y-2014.
3. Dayakar P., Vijay Ruthrapathi G., Prakesh J., Management of bio-medical waste, International Journal of Applied Engineering Research,V-9,I-22,PP-5518-5526,Y-2014.
4. Swaminathan N., Dayakar P., Resource optimization in construction project, International Journal of Applied Engineering Research,V-9,I-22,PP-5546-5551,Y-2014.
5. Venkat Raman K., Dayakar P., Raju K.V.B.,An experimental study on effect of cone diameters in penetration test on sandy soil, International Journal of Civil Engineering and Technology,V-8,I-8,PP-1581-1588,Y-2017.
6. Saritha B., Chockalingam M.P.,Photodgradation of malachite green DYE using TIO2/activated carbon composite,International Journal of Civil Engineering and Technology,V-8,I-8,PP-156-163,Y-2017
7. Shendge R.B., Chockalingam M.P., Saritha B., Ambica A.,Swat modelling for sediment yield: A case study of Ujjani reservoir in Maharashtra, India,International Journal of Civil Engineering and Technology,V-9,I-1,PP-245-252,Y-2018
8. Chockalingam M.P.,Balamurgan V.,Modernisation of an existing urban road-sector in Chennai, a case study report,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1457-1467,Y-2017
9. Saritha B., Chockalingam M.P.,Adsorption study on removal of basic dye by modified coconut shell adsorbent, International Journal of Civil Engineering and Technology,V-8,I-8,PP-1370-1374,Y-2017
10. Saritha B., Chockalingam M.P.,Adsorptive removal of heavy metal chromium from aqueous medium using modified natural adsorbent,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1382-1387,Y-2017
11. Chockalingam M.P., Palanivelraja S.,Retrospective analysis of a theoretical model used for forecasting future air quality near the north Chennai thermal power plant,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1457-1467,Y-2017
12. Saritha B., Chockalingam M.P.,Photodegradation of methylene blue dye in aqueous medium by Fe-AC/TiO2 Composite,Nature Environment and Pollution Technology,V-17,I-4,PP-1259-1265,Y-2018
13. Shendge R.B., Chockalingam M.P., Kaviya B., Ambica A.,Estimates of potential evapotranspiration rates by three methods in upper Bhima Basin, In Maharashtra, India,International Journal of Civil Engineering and Technology,V-9,I-2,PP-475-480,Y-2018
14. Shendge R.B., Chockalingam M.P.,The soil and water assessment tool for Ujjani Reservoir,International Journal of Mechanical Engineering and Technology,V-9,I-2,PP-354-359,Y-2018
15. Shendge R.B., Chockalingam M.P.,A review on soil and water assessment tool,International Journal of Mechanical Engineering and Technology,V-9,I-2,PP-347-353,Y-2018
16. Sachithanandam P., Meikandaan T.P., Srividya T.,Steel framed multi storey residential building analysis and design,International Journal of Applied Engineering Research,V-9,I-22,PP-5527-5529,Y-2014
17. Meikandaan T.P., Ramachandra Murthy A.,Study of damaged RC beams repaired by bonding of CFRP laminates,International Journal of Civil Engineering and Technology,V-8,I-2,PP-470-486,Y-2017
18. Meikandaan T.P., Ramachandra Murthy A.,Retrofitting of reinforced concrete beams using GFRP overlays,International Journal of Civil Engineering and Technology,V-8,I-2,PP-423-439,Y-2017
19. Meikandaan T.P., Ramachandra Murthy A.,Flexural behaviour of RC beam wrapped with GFRP sheets,International Journal of Civil Engineering and Technology,V-8,I-2,PP-452-469,Y-2017
20. Meikandaan T.P., Murthy A.R.,Experimental study on strengthening of rc beams using glass Fiber,International Journal of Civil Engineering and Technology,V-9,I-11,PP-959-965,Y-2018
21. Meikandaan T.P., Hemapriya M.,Use of glass FRP sheets as external flexural reinforcement in RCC Beam,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1485-1501,Y-2017
22. Saraswathy R., Saritha B.,Planning of integrated satellite township at Thirumazhisai,International Journal of Applied Engineering Research,V-9,I-22,PP-5558-5560,Y-2014
23. Saritha B., Ilayaraja K., Eqyaabal Z.,Geo textiles and geo synthetics for soil reinforcement,International Journal of Applied Engineering Research,V-9,I-22,PP-5533-5536,Y-2014
24. Ambica A., Saritha B., Changring G., Singh N B., Rajen M., Salman Md.,Analysis of groundwater quality in and around Tambaram taluk,
- Kancheepuram district,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1362-1369,Y-2017
25. Arunya A., Sarayu K., Ramachandra Murthy A., Iyer N.R.,Enhancement of durability properties of bioconcrete incorporated with nano silica,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1388-1394,Y-2017
26. Ilayaraja K., Krishnamurthy R.R., Jayaprakash M., Velmurugan P.M., Muthuraj S.,Characterization of the 26 December 2004 tsunami deposits in Andaman Islands (Bay of Bengal, India),Environmental Earth Sciences,V-66,I-8,PP-2459-2476,Y-2012
27. Ilayaraja K.,Morphometric parameters of micro watershed in Paravanar sub-basin, Cuddalore District,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1444-1449,Y-2017
28. Ilayaraja K., Singh R.K., Rana N., Chauhan R., Sutradhar N.,Site suitability assessment for residential areas in south Chennai region using remote sensing and GIS techniques,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1468-1475,Y-2017
29. Ilayaraja K., Reza W., Kumar V., Paul S., Chowdhary R.,Estimation of land surface temperature of Chennai metropolitan area using Landsat images,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1450-1456,Y-2017
30. Chitra R.,Experimental study on beam using steel fiber and latex,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1395-1403,Y-2017
31. Chitra R.,Analysis of traffic and management at Kovilambakkam intersection,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1433-1443,Y-2017
32. Aswathy M.,Experimental study on light weight foamed concrete,International Journal of Civil Engineering and Technology,V-8,I-8,PP-1404-1412,Y-2017
33. Aswathy M.,Wastewater treatment using constructed wetland with water lettuce (Eichornia Crasipes),International Journal of Civil Engineering and Technology,V-8,I-8,PP-1413-1421,Y-2017
34. Kiruthiga K., Anandh K.S., Gunasekaran K., Assessment of influencing factors on improving effectiveness and productivity of construction engineers, 2015, International Journal of Applied Engineering Research, V - 10,I - 17,p -13849-13854.

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