

# Determination of Relative Positions of Points Without Establishing Physical Contact With The Object By Concept Of Photogrammetry



Manoj Karagudri, Rambha Thakur, Smeet Faldu, Narendra Kumar Maurya

**Abstract:** Research work done in Terrestrial photogrammetry to calculate and determine various unknown relative positions and parameters of the object. Photogrammetry is one of the surveying concepts of civil engineering to calculate accurate measurements using photographs. Can also be performed without establishing physical contact with the object. Determination of following unknown parameters. The first phase of procedure is: (1.1) Determination of Camera Focal Length. The second phase of procedure is: (2.1) Determination of horizontal distance between the object "A" and the instrument station "M". Second phase also includes (2.2) Determination of horizontal distance between the object "A" and the instrument station "N". The third phase of procedure is: (3.1) Determination of Reduced Level (elevation) of the object point. Initial declaration of pre conditions: The photographs of the object were take from two camera stations M and N with a known measured distance between ground station points "M" and N being 124 meters called as base line. No physical contact was established with the object.

**Keywords:** photogrammetry, relative positions, horizontal angles, horizontal distances, vertical distance, reduced level and elevation, surveying.

## I. INTRODUCTION:

Nature of research work: The paper deals with research project in photogrammetry. The aim of the project is to investigate all the features in detection of relative positioning and filtering of the measurements acquired by the instruments. In addition, intrinsic methodologies were developed and tested for quality assessment. All these investigations will lead to the definition of guidelines for performing research works. The aim and purpose of work is to enhance and favour knowledge exchange and collaboration between international researchers to enhance advancements. In the project there is affinity, concerning the fundamental methodology which has been followed having many ground checks measured.

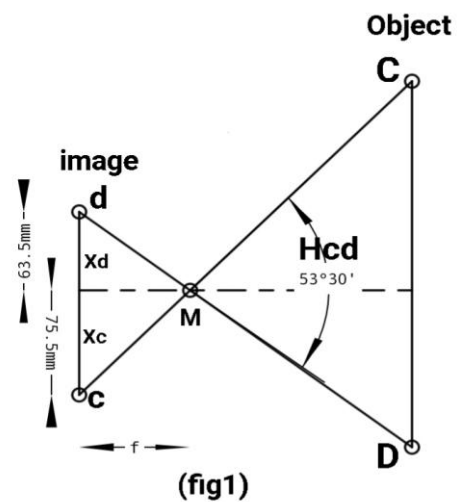
The development of intrinsic methodologies in order to control the quality of the results and the drawing up of a series of rules for the research project. Both artificial and natural ground points were used as photogrammetric control points.

Artificial points obviously obtain a better collimation and an enhanced accuracy. In order to reach its objectives, the project has been divided into three phases, based on the different data sets obtained.

## II. FIRST PHASE OF RESEARCH PROCEDURE:

(1.1) Determination of Camera Focal Length.

The focal length of the lense was calculated by taking one photograph from station (M). Refer figure: (fig1). Two object points (C) and (D) were considered and the angle between them was measured from point (M) by theodolite as 53° degrees 30' minutes. On the photograph the relative positions of two points were noted as 75.5 mm (Xc) to the left and 63.5 mm (Xd) to the right from the principal line (which is the Vertical axis assumed to be Y-axis). Distances along the horizon line are assumed as X-axis measurements. By substitution of these values in the below focal length formula:



$$f = [ (Xc+Xd)/(2 \tan Hcd) ] + \sqrt{ \{ [ (Xc+Xd)/(2 \tan Hcd) ]^2 + (Xc \cdot Xd) \} } \text{---(eq1)}$$

Xc and Xd are the mesured distances in the photograph along X-axis. The relative positions of two points were noted as 75.5 mm to the left and 63.5 mm to the right from the principal line.

Hcd is the horizontal angle measured from point (M) between two points C and D.

Xc=75.5, Xd=63.5, Hcd=53°30',

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\* Correspondence Author

**Manoj Karagudri\***, Assistant Professor, Department of Civil Engineering in various universities in India including, Graphic Era Hill University, Shoolini University, Bahra University, Green Hills Engineering College, Green Hills Engineering College, Bells Institute of Management and Technology.

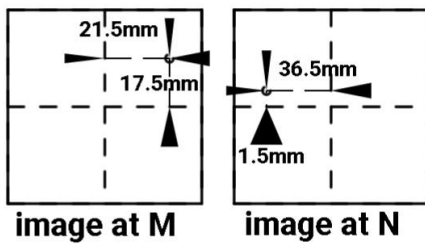
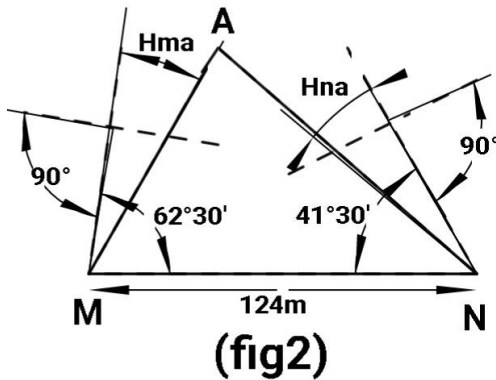
**Rambha Thakur**, Assistant Professor, Department of Civil Engineering, Madhav University, India

**Smeet Faldu**, Assistant Professor, Department of Civil Engineering, Madhav University, India

**Narendra Kumar Maurya**, Ph.D. in Environmental Engineering from Madhav University, India

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$(X_c + X_d) = 139$ ,  $(2 \tan H_{cd}) = 2.703$ ,  
 $[(X_c + X_d) / (2 \tan H_{cd})] = 51.424$ ,  
 $[(X_c + X_d) / (2 \tan H_{cd})]^2 = 2644.428$   
 $(X_c * X_d) = 4794.25$   
 Substituting above values into the equation (eq1).  
 $f = 51.424 + \sqrt{2644.428 + 4794.25}$ ,  $f = 51.424 + \sqrt{7438.678}$ ,  
 $f = 51.424 + 86.248$   
 $f = 137.672 \text{ mm}$  \_\_\_\_\_ (eq2)  
 Thus the focal length (f) of camera was calculated.



**III. SECOND PHASE OF RESEARCH PROCEDURE:**

(2.1) Determination of the relative position of (A) from (M) and (N). The horizontal distance between the object "A" and the instrument station "M". Second phase also includes:  
 (2.2) Determination of horizontal distance between the object "A" and the instrument station "N". To calculate horizontal distances of object from the base line stations. Two photographs were taken from M and N. With the angles of camera axis with the baseline as 62°30' and 41°30' respectively from M and N. Refer figure: (fig2)

In the photograph taken from station M the considered single object point (A) appears 21.5 mm to the right and 17.5 mm above the cross hair lines. In the photograph taken from station N the considered single object point (A) appears at 36.5 mm to the left and 1.5 mm above the cross hairs. The noted elevation and reduced level of the instrument axis at station M was 127.65 m. Considering the photograph taken at M the horizontal angle (Hma) is calculated as follows.

$\tan H_{ma} = \text{opposite/adjacent} = X_a/f$ ,  
 $H_{ma} = \tan^{-1} X_a/f$ ,  $X_a = 21.5 \text{ mm}$ ,  $f = 137.672 \text{ mm}$   
 $H_{ma} = 8.875^\circ$  \_\_\_\_\_ (eq3)

The internal angle of triangle MNA at station M is:  $62^\circ 30' - 8.875^\circ = 53.625^\circ = \text{angle M}$ .

To be noted that in the imaginary triangle MNA, M and N are station points on the baseline and A is the considered object point. Considering the photograph taken at N the horizontal angle (Hna) is calculated as follows.

$\tan H_{na} = \text{opposite/adjacent} = X_a/f$ ,  
 $H_{na} = \tan^{-1} X_a/f$ ,  $X_a = 36.5 \text{ mm}$ ,  $f = 137.672 \text{ mm}$ ,  
 $H_{na} = 14.847^\circ$  \_\_\_\_\_ (eq4)

The internal angle of triangle MNA at station N is:  
 $41^\circ 30' - 14.847^\circ = 26.653^\circ = \text{angle N}$

The internal angle of triangle MNA at station A is:  
 $180^\circ - 26.653^\circ - 53.625^\circ = 99.722^\circ = \text{angle A}$

To calculate the distance of MA from the sine rule:  
 With reference to figure: (fig2)

$MA / \sin N = MN / \sin A$ ,  
 $MA = MN * \sin N / \sin A$   
 $MN = 124 \text{ m}$  (Known distance of baseline)  
 $\sin N = \sin 26.653^\circ = 0.4486$   
 $\sin A = \sin 99.723^\circ = 0.9856$

By substituting the above values.

$MA = 56.439 \text{ m}$  \_\_\_\_\_ (eq4)

To calculate the distance of NA from the sine rule:  
 $NA / \sin M = MN / \sin A$ ,  
 $NA = MN * \sin M / \sin A$

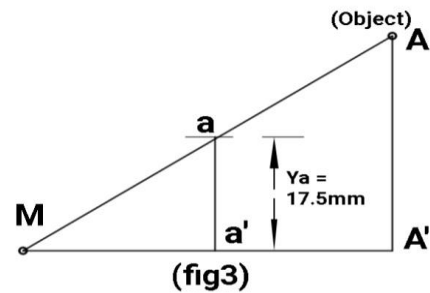
$MN = 124 \text{ m}$  (Known distance of baseline)  
 $\sin M = \sin 53.625^\circ = 0.8052$   
 $\sin A = \sin 99.723^\circ = 0.9856$

By substituting the above values.

$NA = 101.304 \text{ m}$  \_\_\_\_\_ (eq5)

**IV. THIRD PHASE OF RESEARCH PROCEDURE:**

Calculation of elevation (Reduced Level) of object A.  
 $RL \text{ of } A = (RL \text{ of instrument}) + (\text{Height of } A)$   
 Considering A and A' as end points of the height of object A. Refer Figure: (fig3).  
 aa' as the photograph height of A (Ya) along y-axis  
 Measured from the photograph taken from station M is 17.5 mm.



From imaginary triangle AA'M  
 $AA' / aa' = MA' / Ma'$ ,  
 $Ma' = f$  (focal length of the lense),  
 $AA' = aa' * MA' / f$  \_\_\_\_\_ (eq6)

$aa' = 17.5$ ,  $MA' = 56.439 \text{ m}$ ,  $f = 137.672 \text{ mm}$   
 After substituting the values in (eq6)  
 $AA' = 7.174 \text{ m}$  (Height of the object A from horizontal plane.)

Reduced Level of A =  
 (RL of instrument at M) + (Height of AA')

The noted elevation of the instrument axis at station M was 127.65 m.  
 $= 127.65 + 7.174$   
 $= 134.824 \text{ m}$

The calculated Reduced Level (Elevation) of A is 134.824 meters.

**V. RESULT:**

The calculated research results were as follows:

From First phase of research procedure:

(1) The Camera Focal Length was determined as: 137.672 mm

From Second phase of research procedure:

(2) The horizontal distance between the object (A) and the instrument station (M) was determined as 56.439 meters.

(3) The horizontal distance between the object (A) and the instrument station (N) was determined as 101.304 meters.

From Third phase of research procedure:

(4) The Reduced Level (elevation) of the object point A was determined as 134.824 meters.

**Table: Showing The Calculated Results**

Name	Description	Value
Angle M	Horizontal Angle M in triangle MNA	53.625°
Angle N	Horizontal Angle N in triangle MNA	26.653°
Angle A	Horizontal Angle A in triangle MNA	99.722°
MN	Horizontal distance between points M and N	124 m
MA	Horizontal distance between points M and A.	56.439 m
NA	Horizontal distance between N and A.	101.304 m
f	Focal length of the camera.	137.672 mm
AA'	Height of the object A from the horizontal plane.	7.174 m
RL of A	Reduced level of the object A.	134.824 m

**CONCLUSION:**

Advantages are that there is no need for person to hold the object staff or measuring tape. Helps reduce physical effort and to reduce number of helping people. Thus reduce in labour cost and reduce in labour time consumption. Disadvantages are that the difficulties are encouraged if any obstacles are intercepted blocking the image of objects. Careful selection of camera stations to get visible images of objects is necessary.


**REFERENCES:**

- Luhmann, T, Robson, S, Kyle, S, & Harley, I. (2006). Close range photogrammetry: principles, methods and applications. Scotland, UK: Whittles Publishing.
- Luhmann, T. (2010). Close range photogrammetry for industrial applications. ISPRS Journal of Photogrammetry and Remote Sensing, 65(6), 558–569.
- Badekas, John. 1975. Photogrammetric Surveys of Monuments and Sites”, International Symposium on Photogrammetric Surveys of Monuments and Sites. Amsterdam, Ed.1975.
- Goel Salil, Yadav Manohar, Singh Anjani K., and Lohani Bharat, 2013. An indigenous development of land based mobile mapping system”, India Geospatial Forum, Hyderabad, 22-24, January.
- Dai, F, & Lu, M. (2010). Assessing the accuracy of applying photogrammetry to take geometric measurements on building products. Journal of Construction Engineering and Management, 136(2), 242–250. Reston, VA: ASCE
- Blachut, TJ, & Burkhardt, R. (1989). Chapter 1. In Historical development of photogrammetric methods and instruments. Falls Church, VA: American Society for Photogrammetry and Remote Sensing.

- Bhatla, A, Choe, SY, Fierro, O, & Leite, F. (2012). Evaluation of accuracy of as-built 3D modeling from photos taken by hand held digital cameras. Automation in Construction, 28, 116–127.
- González-Jorge, H, Riveiro, B, Armesto, J, & Arias, P. (2011). Verification artifact for photogrammetric measurement systems. Optical Engineering, 50(7), 073603.
- F. Samadzadegan a, P. Ramzi a, T. Schenk. 2008. AERIAL TRIANGULATION OF DIGITAL AERIAL IMAGERY USING HYBRID FEATURES. The International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences. Vol. XXXVII. Part B1. Beijing.
- Fei Dai, Youyi Feng, and Ryan Hough. Dai et al. 2014. Photogrammetric error sources and impacts on modeling and surveying in construction engineering applications. Visualization in Engineering, 2:2 Page 5 of 14 <http://www.viejournal.com/content/2/1/2>
- Surendra Pal Singh, Kamal Jain, V. Ravibabu Mandla. 2014. A NEW APPROACH TOWARDS IMAGE BASED VIRTUAL 3D CITY MODELING BY USING CLOSE RANGE PHOTOGRAMMETRY. ISPRS Annals of the Photogrammetry, Remote Sensing and Spatial Information Sciences, Volume II-5, 2014 ISPRS Technical Commission V Symposium, 23 – 25 June 2014. Riva del Garda, Italy.
- Surendra Pal Singh, Kamal Jain, V.Ravibabu Mandla. 2013. Virtual 3D Campus Modeling by Using Close Range Photogrammetry. American Journal of Civil Engineering and Architecture, 2013, Vol. 1, No. 6, 200-205.
- Galetto, R., Casella, V., Spalla, A., Franzini, M., 2004. An Italian research project on direct photogrammetry. Proceedings of XXth ISPRS Congress, Istanbul, Turkey, published on DVD.
- M. Saadatesherst, A.H. Hashempourb, M. Hasanloua. 2015. UAV PHOTOGRAMMETRY: A PRACTICAL SOLUTION FOR CHALLENGING MAPPING PROJECTS. The International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences, Volume XL-1/W5, 2015 International Conference on Sensors & Models in Remote Sensing & Photogrammetry, 23–25 Nov 2015, Kish Island, Iran
- Anoop Sattinenia and James Hindman. 2017. Photogrammetric Models using Oblique Aerial Imagery for Construction Site Surveying. 34th International Symposium on Automation and Robotics in Construction (ISARC 2017).
- Schenk, T., B. Csathó, and D.C. Lee, 1999. Quality control issues of airborne laser ranging data and accuracy study in an urban area. International Archives of Photogrammetry and Remote Sensing, Vol. 32, Part 3-W14, pp. 101-108.
- Schenk, T, S. Seo, and B. Csathó, 2001. Accuracy study of airborne laser scanning data with photogrammetry. International Archives of Photogrammetry and Remote Sensing, Vol. 34, Part 3-W4, pp. 113-118.
- Jian Kang, Marco Körner, Yuanyuan Wang, Hannes Taubenböck, Xiao Xiang Zhu. 2018. Building instance classification using street view images. ISPRS Journal of Photogrammetry and Remote Sensing 145 (2018) 44–59.
- Slama, C.C., ed., 1980. Manual of Photogrammetry, Fourth Edition. American Society of Photogrammetry, Falls Church, Virginia, Ch. II, pp. 37-101.
- Anuar Ahmad, Lawali Rabi. 2011. Generation of Three Dimensional Model of Building using Photogrammetric Technique”, IEEE, 7th International Colloquium on Signal Processing and its Applications, 2011.
- Baltsavias, E. (1999). A comparison between photogrammetry and laser scanning. ISPRS Journal of Photogrammetry and Remote Sensing, 54(2–3), 83–94.
- Luhmann, T. (2009). Precision potential of photogrammetric 6DOF pose estimation with a single camera. ISPRS Journal of Photogrammetry and Remote Sensing, 64(3), 275–284.
- Luhmann Thomas, and Tecklenburg Werner, 2004, “3D object reconstruction from multiple-station panorama imagery”, International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences 34, No. 5/W16: 8.
- Fryer, J. (1996). Camera calibration. In Atkinson (Ed.),
- Close-range photogrammetry and machine vision (pp. 156–179). UK: Whittles Publishing. Golparvar-Fard, M, Bohn, J, Teizer, J, Savarese, S, & Peña-Mora, F. (2011). Evaluation of image-based modeling and laser scanning accuracy for emerging automated performance monitoring techniques. Automation in Construction, 20(8), 1143–1155.

25. Dai, F, & Lu, M. (2008). Photo-based 3D modeling of construction resources for visualization of operations simulation: case of modeling a precast facade. In SJ Mason, R Hill, L Moench, & O Rose (Eds.), Proceedings of the 2008 winter simulation conference (pp. 2439–2446). Miami, Florida: IEEE.

**AUTHORS PROFILE**

	<p>Mr. Manoj Karagudri, born in 1972. He has completed his graduation in Civil Engineering in 1996 from Karnatak University Dharwad, India with first class. Post Graduation (M-TECH) in Highway Civil Engineering in 2019 from Shri Venkateshwara University, Uttar Pradesh, India with first class. His professional experience is 21 years in Training/Teaching, Execution, Management, Information Technology, Software Development. He has experience of 08 years as Assistant Professor, Department of Civil Engineering in various universities in India including, Graphic Era Hill University, Shoolini University, Bahra University, Green Hills Engineering College, Bells Institute of Management and Technology. Email: prayersofhope@gmail.com</p>
	<p>Ms. Rambha Thakur, born in 1992. She has completed her graduation in Civil Engineering in 2015 from Shoolini University, India with first class. Post Graduation in Structural Engineering in 2017 from A.P.Goyal University, India with first class. At present, she is pursuing her Ph.D. in Structural Engineering from Madhav University, India. Her professional experience is 02 years as an Assistant Professor, Department of Civil Engineering, Madhav University, India. She has 02 publishing to her credit and guided 03 postgraduate projects. Email: rambhathakur786@gmail.com</p>
	<p>Mr. Smeet Faldu, born in 1993. He has completed his graduation in Civil Engineering in 2015 from Rajasthan Technical University, India with first class, Honours. Post Graduation in Structural Engineering in 2018 from Charotar University of Science and Technology, India with first class, Distinction. At present, He is pursuing his Ph.D. in Structural Engineering from Madhav University, India. His professional experience is 02 years as an Assistant Professor, Department of Civil Engineering, Madhav University, India and 01 year as a Junior Engineer, Vishwa Infrastructure, India. He has 04 publishing to his credit and guided 02 postgraduate projects. Email: smitfaldu4521@gmail.com</p>
	<p>Mr. Narendra Kumar Maurya, born in 1990. He has completed his graduation in Civil Engineering in 2013 from Rajasthan Technical University, India with first class. Post Graduation in Environmental Engineering in 2018 from J.N.V.U, India. At present, He is pursuing his Ph.D. in Environmental Engineering from Madhav University, India. His professional experience is 03 years as an Assistant Professor. He has 04 publishing to his credit. Email: er.narendramaurya@gmail.com</p>