# Columns with Different Cross Section using ANSYS Work Bench

# Prabhu. L, Aravindan. V, Amal Joseph, Edwin Joseph, Abhijith V R

Abstract: In this paper we are going to analyze the strength of material and deflection due to applying load on the column. The column of different materials will be cross sectioned and will present the difference of strength of materials by ANSYS workbench. The strength of material will depend upon the young's modulus and moment of inertia of the metal plate. We use different size of metal plates such as I-section, circular bar, rectangular bar and square bar for a specific material to know the strength of material when a load applied on the cross sections. In this paper we taken a condition that one side of column is fixed and another side is free when load is applying on the column.

Keywords: ANSYS Workbench, Column, Crippling Load, Buckling.

### I. INTRODUCTION

The strength of material and deflection due to applying load on the column will be analyzed using the following numerical solution,

$$P Cr = \frac{\pi^2 \varepsilon I}{L_c^2}$$

Where

Pcr = Crippling load (N)

 $\epsilon$  = Young's Modulus of Used Material (N/m2) I = Moment of Inertia (m4)

Lc = Characteristics Length (m)

The strength of material will depend upon the young's modulus and moment of inertia of the metal plate[1]. We use different size of metal plates such as I-section, circular bar, rectangular bar and square bar for a specific material to know the strength of material when a load applied on the cross sectioned columns[2]. The above formula is used for the

#### Revised Manuscript Received on December 16, 2019.

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condition when the load applied openly on the columns, but in this analysis one side of column is fixed and another side is free when load is applied on the column[3]. Then the above formula changed as:-

$$P Cr = \frac{0.25\pi^2 \varepsilon I}{L_c^2}$$

Where

Pcr = Crippling load (N)

E = Young's Modulus of Used Material (N/m2) I = Moment of Inertia (m4)

Lc = Characteristics Length (m)

We use only concrete material in the form of different type of columns[4]. The young's modulus of concrete is  $3\times1010$  N/m2 and moment of inertia varies according to different size of the material and we take 3 meter characteristics length of the columns[5].

#### A. Columns

Column is a vertical member which is having the effective length more than three times of its least lateral dimension carrying compressive loads.

Based on shape columns can be classified into Square, Rectangular, and Circular, L-Type, T-Type, V-Type, Hexagon and Arch type as shown in Fig. 1 to 8.

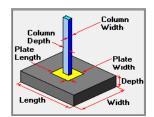


Fig. 1.square column.



Fig. 2. Rectangular column.





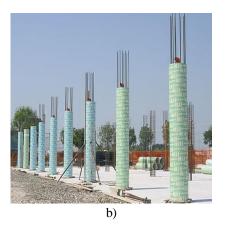


Fig. 3.a) & b)Circular Column.



L-type column.



Fig. 4.T-type column.



Fig. 5.V-type column.



Fig. 6.Hexagonal column.



Fig. 7.Arch type column.

Based on type of reinforcement, columns can be classified to Tied Column, Spiral Column and Composite Column as shown in Fig. 9.

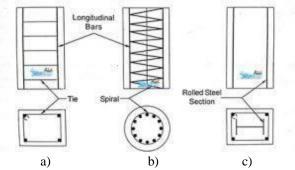


Fig. 8.(a) Tied Column (b) Spiral Column (c) composite Column .

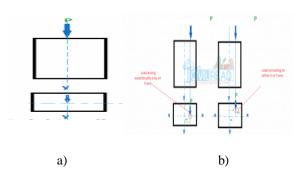


Fig. 9. (a) Axially loaded column , (b) Eccentrically loaded column .

Based on the type of loading columns can be classified to Axially loaded column and Eccentrically loaded column as shown in Fig. 10.Similarly the linked column is shown in the figure 11 and neck column shown in the figure 12.



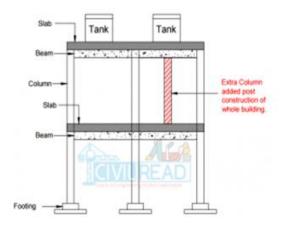


Fig. 10. Stub Column/ Floating column / Linked Column.

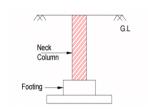


Fig. 11. Neck column.

#### II. MATERIALS AND METHODS

Numerical analysis were carried out for the I Section, Rectangular and circular beam shown in the figure 13-15.

## A. I-Section

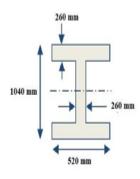


Fig. 12. 13 I-Section.

## Calculation:-

 $\rho$ =2300 kg/m<sup>3</sup> E=3x10<sup>10</sup> N/m<sup>2</sup>

 $I_{xx} = 45697601536 \text{ mm}^4$ 

 $I_{yy} = 6854640128 \ mm^4$ 

We take one side fixed and one side free condition, then  $n{=}0.25\;N$ 

$$P_{C_r} = n \frac{\pi^2 E I}{L_c^2}$$

$$\begin{split} P_{C_r} &= \frac{0.25\pi^2 \times 3 \times 10^{10} \times 6854640128 \times 10^{-12}}{3^2} \\ P_{C_r} &= \frac{7.402203301 \times 10^{10} \times 6854640128 \times 10^{-12}}{3^2} \\ P_{C_r} &= \frac{2.4674011 \times 3 \times 10^{10} \times 6854640128 \times 10^{-12}}{3^2} \\ P_{C_r} &= 28.747 \times 10^6 \end{split}$$

#### B. Circular Beam

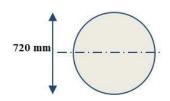


Fig. 13. circular beam.

$$\rho=2300 \text{ kg/m}^3$$

$$E=3x10^{10} \text{ N/m}^2$$

$$I_{xx}=13191672832 \text{mm}^4$$

$$I_{yy}=13191672832 \text{ mm}^4$$

$$P_{C_{r}}=0.25 \frac{\pi^2 E I}{L_c^2}$$

$$P_{c_r}=\frac{0.25\pi^2 \times 3 \times 10^{10} \times 13191672832 \times 10^{-12}}{3^2}$$

$$P_{c_r}=\frac{7.4022 \times 10^{10} \times 13191672832 \times 10^{-12}}{3^2}$$

$$P_{C_r}=108.49 \times 10^6$$

$$P_{c_r}=\frac{9.7647444 \times 10^{10} \times 10^{10} \times 10^{-12}}{3^2}$$

## C. Rectangular Beam

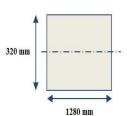


Fig. 14. Rectangular Section.

 $\begin{array}{l} \rho = \! 2300 \text{ kg/m}^3 \\ E \! = \! 3x10^{10} \text{ N/m}^2 \\ I_{xx} = 55924051948 \text{ mm}^4 \\ I_{yy} = 3495253248 \text{ mm}^4 \end{array}$ 

One side fixed and one side free condition we take



$$P_{C_r} = \frac{0.25\pi^2 \times 3 \times 10^{10} \times 3495253248 \times 10^{-12}}{3^2}$$

$$P_{c_r} = \frac{7.4022 \times 10^{10} \times 3495253248 \times 10^{-12}}{9}$$

$$P_{c_r} = \frac{7.4022 \times 10^{10} \times 3495253248 \times 10^{-12}}{9}$$

$$P_{C_r} = 28.747 \times 10^6$$

### III. RESULT AND DISCUSSION

## A. I-Section

Figure 16 shows the ANSYS model for I-section

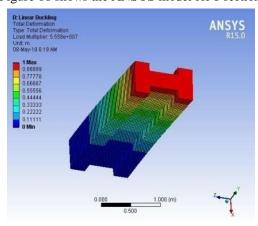


Fig. 15. I-Section.

Simulation of I-Section Crippling load of I-Section is =55.58×106 N.

#### B. Circular Beam

Figure 16 shows the ANSYS model for circular beam.

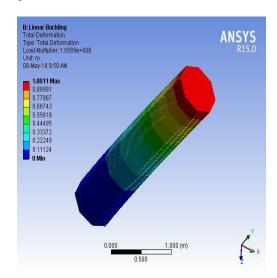


Fig. 16. Simulation of Circular Beam.

Crippling load of Circular Beam is =105.69×106 N.

#### C. Rectangular Beam

Figure 18 shows the ANSYS model for Rectangular beam.

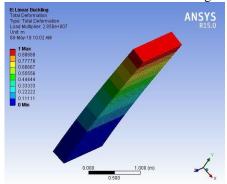


Fig. 17. Simulation of Rectangular Beam.

Crippling load of Rectangular Beam is  $=28.56 \times 106 \text{ N}$ .

Table- I: Comparison and Discussion

Types of Beams	Numerical Analysis	Simulation Analysis	% Error
I-Section Beam	$P_{C_r} = 56.337 \times 10^6 \text{ N}$	P <sub>C<sub>r</sub>=55.58×106</sub> N	1.343
Circular Beam	$P_{C_r} = 108.49 \times 10^6 \text{ N}$	P <sub>C<sub>r</sub></sub> =105.69×106 N	2.580
Rectangular Beam	$P_{c_r} = 28.747 \times 10^6 \text{ N}$	P <sub>C<sub>r</sub>=28.56×106</sub> N	0.650

Consider a circular and a rectangular column of side D, D\*B. The cross sectional area of circular column will be higher when compared to rectangular columns which is shown in the figure 19. Also, the moment of interia of the circular columns will be much higher than that of the rectangular/square columns. Thereby, less dangerous towards buckling. Even in terms of reinforcement,

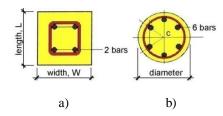


Fig. 18. Cross sectional area of a)Rctangular b)Circular

Min. no. of bars in rectangular column is 4. Whereas, min. no. of bars in a circular column is 6.

#### IV. CONCLUSION

• Due to their Slenderness ratios, circular cross section columns are better.



- Rectangular cross-section is prominent for buckling where circular cross section resists it to maximum extent.
- Circular columns do not have weak corners and stress concentration is nil.
- Shear reinforcement having spiral forms gives better result than any other form of ties.
- As Circular cross section does not have any folds, hence load can be distributed evenly across.
- Rectangular cross section is more prominent to bending compare to circular cross section.

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