

Vibration Analysis on a Thin Walled Structure

Muttangi Sushma, D.Mahesh Kumar, N.Madhavi



Abstract: A vibration analysis performed on thin walled open beam section. The influence of the vibration is studied for different thickness and different critical load conditions. A FEM model is applied to the thin wall sections like beams. For the different young's modulus, critical load and thickness of the flange the mode shapes are analyzed. The free vibrations of the channel unsymmetrical thin-walled beam of length 80, breadth of the flange is $37.81e-3$, thickness of the flange, over all lengths of the flange at $\lambda = (7.5, 10, 12.5, 15, 17.5, 20)$, for $e = 10, 20, 30, 40, G =$ rigidity of modulus also been analyzed..

Keywords: vibration analysis, critical loads, structure, thin walled beams.

I. INTRODUCTION TO THIN WALL BEAMS

An aircraft's structural elements consist primarily of thin plates which are stiffened by rib and stringer arrangements. Thin plates (thin sections/ thin walled structures) are buckled under relatively small compressive loads and so must be stiffened to prevent this.

To determine the thin plates buckling loads in isolation is relatively straightforward but when the stiffeners are attached to the skin, then the problem becomes complex and the solutions relies on an empirical solution. The buckling of the thin plates is a phenomenon in which it may lead to instability and failure of the aircraft components; hence the buckling phenomenon on thin plate finite element analysis is analyzed by ANSYS software.

The different types of thin walled structures or thin sections are: T, C, L, I, Z section. These sections are used in the construction of an aircraft; they are used primarily in the construction of the fuselage.

II. FINITE ELEMENT METHOD

The FEA (Finite Element Analysis) is a numerical method for solving mathematical, engineering and physics problems. Used for complicated geometries, loadings, and material properties where analytical solutions is difficult to obtained.

Two methods of FEM are

1. Force method
2. Displacement method

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In the forced method, the set of equations are related to the displacement points.

- In the stiffness method, the set of equations are the equilibrium equations relating displacements of points.
- Rayleigh-Ritz is a displacement method working on energy principle where equations can be obtained in matrix form.

III. DESIGN SPECIFICATIONS

This journal uses double-blind review process, which means that both the reviewer (s) and author (s) identities concealed from the reviewers, and vice versa, throughout the review process. All submitted manuscripts are reviewed by three reviewer one from India and rest two from overseas. There should be proper comments of the reviewers for the purpose of acceptance/ rejection. There should be minimum 01 to 02 week time window for it.

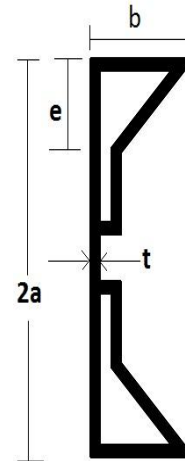


Fig. 1.C-SECTION

A.DIMENSIONS OF THE CROSS SECTIONS

Length of the beam=h

Height of the cross section $H=2xa$

Breadth f the cross section=b

Thickness of the cross section=t

Height of the flange section=e

Case1: λ and **Mcr** values for different thicknesses (t) at $e=10$

E= 10e-3				
$\lambda = L/H$	t= 0.6	t=1.10	t=2.0	t= 3.14

Case 2: λ and **Mcr** values for different thicknesses at $e=20$

E= 20e-3				
$\lambda=7.5$	t= 0.6	t=1.10	t=2.0	t= 3.14

IV. RESULTS & DISCUSSION

CASE:1-Modes For $e=10$, thickness (t)= 0.6 , at $\lambda=(7.5, 10,12.5,15,17.5,20)$

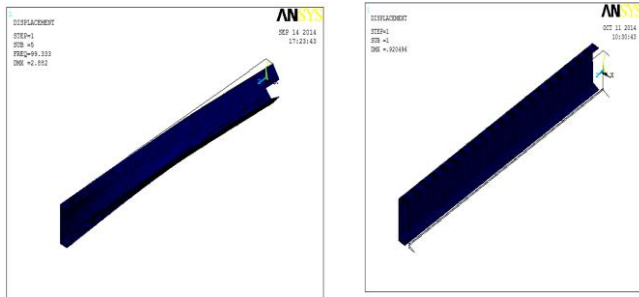
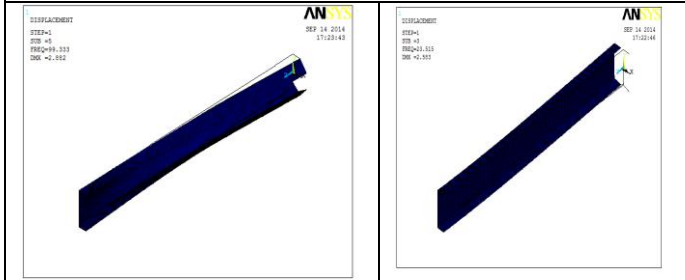
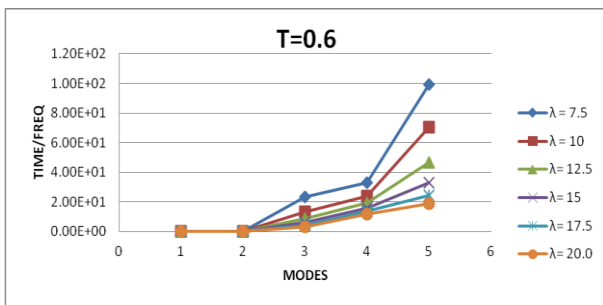


Table for modes and frequencies values for all λ values at $e=10,t=0.6$

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E= 10e-3, t=0.6						
	$\lambda = 7.5$	$\lambda = 10$	$\lambda = 12.5$	$\lambda = 15$	$\lambda = 17.5$	$\lambda = 20$
Mode= 1	Freq= 0.2839	0.2839	0.1858	0.3219	0.3219	0.2399
Mode= 2	0.4293	0.3219	0.3219	0.4156	0.3559	0.3219
Mode= 3	23.515	13.313	8.5391	5.9357	4.3631	3.3415
Mode= 4	33.222	24.112	19.210	15.953	13.552	11.685
Mode= 5	99.333	70.648	46.517	32.860	24.486	19.028



Graph for modes vs frequencies for all λ values at $e=10, t=0.6$

The free vibrations of the channel unsymmetrical thin-walled beam of length $a= 80$, $b =$ breadth of the flange ($37.81e-3$), thickness of the flange at ($t=0.6$), over all lengths of the flange at $\lambda = (7.5, 10,12.5,15,17.5,20)$, for $e = 10,G=$ rigidity of modulus (1),has been studied in this figure.

CASE:2-Modes for $e=10$, thickness(t)= 1.10 , $\lambda=(7.5, 10,12.5,15,17.5,20)$

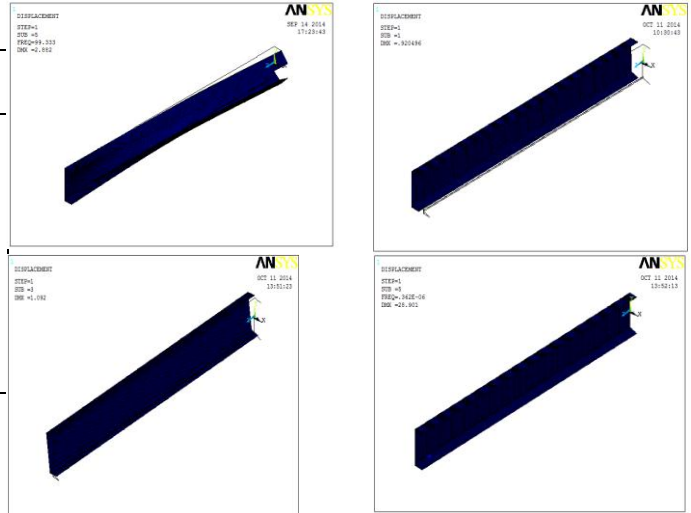
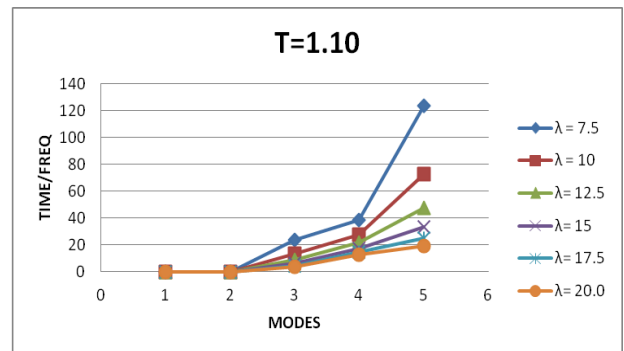


Table for modes and frequencies values for all λ values at $e=10, t=1.10$

E= 10e-3, t=1.10						
	$\lambda = 7.5$	$\lambda = 10$	$\lambda = 12.5$	$\lambda = 15$	$\lambda = 17.5$	$\lambda = 20$
Mode= 1	Freq= 0.000	0.18	0	0.18	0.18	0.23
Mode= 2	999	396	999	589	99	198
Mode= 3	23.6	13.3	8.54	5.93	4.36	3.34
Mode= 4	38.4	27.6	21.5	17.4	14.6	12.4
Mode= 5	123.	72.4	47.2	33.2	24.7	19.2
	8	15	31	58	74	82



Graph for modes vs frequencies for all λ values at $e=10, t=1.10$

V.CONCLUSION

The free vibrations of the channel unsymmetrical thin-walled beam of length $a= 80$, $b =$ breadth of the flange ($37.81e-3$), thickness of the flange at ($t=0.6$), over all lengths of the flange at $\lambda = (7.5, 10, 12.5, 15, 17.5, 20)$, for $e = 10,G=$ rigidity of modulus (1),has been studied in case 1 The free vibrations of the channel unsymmetrical thin-walled beam of length $a= 80$, $b =$ breadth of the flange ($37.81e-3$), thickness of the flange at ($t=1.10$), over all lengths of the flange at $\lambda = (7.5, 10, 12.5, 15, 17.5, 20)$, for $e = 10,G=$ rigidity of modulus (1),has been studied in case 2.

REFERENCES

1. K. Chen, Linear Networks and Systems (Book style).Belmont, CA: Wadsworth, 1993, pp. 123–135.
2. Hl. Timoshenko S, Young D. Vibration problems in engineering. 3rd ed. Princeton, NJ: Van Nostrand; 1968
3. 2. Arpaci A. Bozdog E. On free vibration analysis of thin-walled beams with nonsymmetrical open cross sections. Computers and structures 2002; 80:691-5
4. Tanaka M, Bercin A. Free vibration solution for uniform beams of nonsymmetrical cross section using Mathematica. Computers and structures 1999; 71: 1-8
5. ProkicA.On fivefold coupled vibrations of Timoshenko thin-walled beams. Engineering structures 2006; 28: 54-62
6. Chen HH, Hsiao KM. Quadruply coupled linear free vibrations of thin walled beams with a generic open section. Engineering structures 2008; 30:1319-34
7. Yoon KY, Kang YJ, Choi YJ, Park NH. Free vibration analysis of horizontally curved steel I-girder bridges. Thin-walled structures2005; 43: 679-99
8. Piovan M, CortinezV.Mechanics of Thin-walled curved beams made of composite materials, allowing for shear deformability. Thin-walled structures 2007; 759-89
9. Ebner A, Billington D. Steady state vibrations of damped Timoshenko beams.Journal of the Structural Division, ASCE 1968; 94: 737-60
10. Ambrosini D, Riera JD, Danesi RF. Dynamic analysis of thin- walled and variable open section beams with shear flexibility. International Journal for Numerical Methods in Engineering 1995; 38: 2867-85
11. Ambrosini D, Riera JD, Danesi RF. A modified Vlasov theory for dynamic analysisof thin-walled and variable open section beams. Engineering Structures 2000; 22: 890-900
12. Gere J, Lin Y. Coupled vibrations of thin- walled beams of open cross section.Journal of Applied Mechanics, ASME 1958; 25: 373-8
13. Aggarwal H, Cranch E. A theory of torsional and coupled bending torsional waves in thin- walled open section beams.Journal of Applied Mechanics, ASME 1967; 34: 337-43
14. Yaman Y. Vibration of open-section channel: a coupled flexural and torsional wave analysis. Journal of Sound Vibration 1997; 204: 131-58
15. Ali Hasan S, Barr A. Linear vibration of thin-walled beams of equal angle section. Journal of Sound and Vibration 1974; 32:3-23.
16. Bishop RE, Cannon SM, Miao S. On coupled bending and torsional vibration of uniform beams. Journal of Sound and Vibration 1989; 131: 457-64
17. “Buckling analysis of thin wall stiffened composite panels”, Patabhi N Madhavi, Y.B.SudhirSastry, R.Budarapu. 2015/1, Volume 96, Issue 5
18. Buckling Analysis Of Stiffened Composite Panels For Different Ply Orientations,N.Madhavi,K.Sreelakshmi,Sudhakar Atchyutuni
19. Damage Analysis Of Low Speed Impact On Composite Materials, Vamsi V K Veeranjaneyulu,M S N Gupta,Dhanajayan, (IJCIET), Volume 8, Issue 5.

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